INFLUENCE OF REACTION TIMES AND ANTICIPATION ON THE STABILITY OF VEHICULAR TRAFFIC FLOW

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Abstract: We investigate two causes for the instability of traffic flow: The time lag caused by finite accelerations of the vehicles, and the delay caused by the finite reaction times of the drivers. Furthermore, we simulate to which degree drivers may compensate for these delays by looking several vehicles ahead and anticipate future traffic situations. Since vehicular traffic flow is a multi-particle system with many degrees of freedom, two concepts of linear stability have to be considered: Local stability of a car following a leader that drives at constant velocity, and string (chain) stability of a “platoon” of several vehicles following each other. Typically, string stability is a much more restrictive criterion than local stability. We simulate both types of stability with the human driver model (HDM)[M. Treiber et al., Physica A, Vol. 360 (1), 71-88 (2006)], which includes all the features above. We found several remarkable results: (i) with a suitable anticipation, we obtained string stability for reaction times exceeding the ”safe time headway”, which, to date, has not yet been obtained for any other car-following model; (ii) parameter changes that destabilize the model variant with zero reaction time may stabilize the model with finite reaction times and vice versa, (iii) distributed reaction times (every driver has a different reaction time) can stabilize the system compared to drivers with identical reaction times that are equal to the mean.

Keywords: Car-following model, microscopic traffic simulation, human reaction time, time-delayed system, local and string stability.

1. INTRODUCTION

Similarly to other feedback control systems, the instability of traffic flow is caused by the delay the system needs to respond to a certain action of a controller (Brogan, 1991; Isidori, 1995). More specifically, the controllers are the drivers, the quantity to be controlled is the velocity of the own vehicle, or the distance to the preceding vehicle, the input stimuli are the observed distances and velocities, respectively, and the actions to reach desired velocities or distances consist in accelerat-

ing or braking (we do not consider lane changes here).

While the modelling of human driving behavior is a controversial topic in traffic science (Brackstone and McDonald, 1999; Holland, 1998; Helbing, 2001), it is obvious, that an essential feature of human (in contrast to automated) drivers is a considerable reaction time, which is a consequence of the physiological aspects of sensing, perceiving, deciding, and performing an action (Shiffrin and Schneider, 1977). This complex reaction time $T'$ is of the order of $1.2$ s (Green, 2000). In addition,
it varies strongly between different drivers (age, gender), different stimuli, and different studies (cf. the review of human perception-brake reaction time studies (Green, 2000)). Remarkably, in dense (not yet congested) traffic, the modal value in the time headway distribution (which is the most probable value) on Dutch or German freeways are around 0.9 s (Tilch and Helbing, 2000; Knoespe et al., 2002), i.e., below the average value of the reaction time. The most obvious means to model reaction times is to introduce a dead time (time delay) \( T' \) between the accelerating or braking action of the driver, and the input stimuli to which a driver reacts (Newell, 1961; Bando et al., 1998; Davis, 2002). However, another source of time delays is the finite acceleration capability of the vehicles: The desired velocity is the integral of the action, i.e., the acceleration. To obtain the desired distance, one even has to integrate once more.

In the context of feedback control theory, the task of following a single vehicle can be modelled by a nonlinear controller containing a nonlinear gain function, a dead-time or delay element, and one or two I-elements. The nonlinear gain function is represented by a conventional car-following model giving the instantaneous acceleration as a function of the velocity of the own and the preceding vehicle, and the gap between the vehicles. In this work, we will use the intelligent driver model (IDM) (Treiber et al., 2000) to be described below for this purpose. Basically, the IDM describes automated driving, sometimes called ‘adaptive cruise control’ (ACC) (Kesting et al., 2006). Unlike machines, human drivers routinely scan the traffic situation several vehicles ahead and anticipate future traffic situations leading, in turn, to an increased stability. In the language of feedback control systems, additional (nonlinear) derivative elements are incorporated into the control path.

For determining the linear local stability, one can apply standard methods of control theory to the linearized system yielding an upper limit of the reaction time \( T' \), which decreases with the sensitivity of the car-following model, i.e., how much it accelerates (decelerates) if the actual distance is too large (small).

However, it is well known in traffic theory for car-following models with zero reaction time (Treiber et al., 2000; Helbing, 2001), and also for microscopic models (Treiber et al., 1999), that small sensitivities (acceleration capabilities) increase the linear string instability if a platoon of several vehicles is considered, i.e., the perturbations will amplify while propagating downstream the chain of vehicles. Typically, string or collective stability is a much more restrictive criterion than local stability. The regime of string instability can be further divided into a region of convective instability where perturbations grow but finally are convected out of the system (cf., e.g., Fig. 4 below), and a region of absolute instability.

In this contribution, we investigate, by means of simulation, the influence of (i) reaction times, (ii) acceleration capabilities, (iii) temporal anticipation, and (iv) multi-vehicle look-ahead on the stability of traffic flow. We put traffic dynamics in the context of feedback control theory and discuss how the influencing factors mentioned above change local stability, string stability, and the limits where the traffic flow is accident-free (which is an inherently nonlinear problem).

In Sec. 2, we present the models used for the simulation. The intelligent-driver model (IDM) (Treiber et al., 2000) will be used as instantaneous nonlinear controller representing the characteristics of automated driving. Its sensitivity is characterized by the acceleration parameter \( a \). The recently proposed human driver model (HDM) (Treiber et al., 2006) implements the human-specific properties (reaction times and anticipations) in a systematic way.

In Section 3, we give the results and show how each of the effects mentioned above influences the traffic dynamics. With a suitable anticipation, we obtained string stability for reaction times exceeding the “safe time headway”, which, to date, has not yet been obtained for any other car-following model. Furthermore, we show how the different influences of reaction time and acceleration capability on local and string stability lead to an optimal range of the acceleration parameter \( a \) rather than a lower limit as proposed in the literature up to now. Finally, we simulate, for the first time, distributed (i.e., varying) reaction times.

In the concluding Section 4 we discuss the results in the context of feedback control theory.

2. MICROSCOPIC TRAFFIC MODEL WITH TIME DELAY AND ANTICIPATION

Most microscopic traffic models describe the instantaneous acceleration and deceleration of each individual ‘driver-vehicle unit’ as a function of the distance and velocity difference to the vehicle in front and on the own velocity (Helbing, 2001). The subclass of time-continuous micromodels (car-following models) is of the general form

\[
\frac{dv_a}{dt} = a \text{mic}(s_a, v_a, \Delta v_a),
\]

where the own velocity \( v_a \), the net distance \( s_a \), and the velocity difference \( \Delta v_a \) to the leading vehicle serve as stimuli determining the acceleration \( \dot{v}_\text{mic} \). This class of basic models is characterized
by (i) instantaneous reaction, (ii) reaction only to the immediate predecessor, and (iii) infinitely exact estimating capabilities of drivers regarding the input stimuli $s$, $v$, and $\Delta v$, which also means that there are no fluctuations. In some sense, such models describe driving behavior similar to adaptive cruise control (ACC) systems (Kesting et al., 2006).

In the context of control theory, the acceleration is the action to bring

- the own velocity $v_\alpha$ towards the desired velocity $v_\alpha^*$ if there is no obstruction from other vehicles, and to the velocity $v_{\alpha-1}$ of the predecessor otherwise,
- the observed distance $s_\alpha$ towards the equilibrium distance $s_e(v_{\alpha-1})$. Of course, this condition is only relevant in case of obstruction. For models of the form (1), the equilibrium distance function $s_e(v)$ is given by

$$d^{\text{mic}}(s, v, 0) = 0. \quad (2)$$

In general, the control function $\dot{v}_{\text{mic}}$ is strongly nonlinear, and there is a smooth transition from the control targets for unobstructed traffic to that of obstructed traffic. Notice that ‘obstructed traffic’ (i.e., it is not possible to drive at the desired velocity) does not necessarily mean ‘congested traffic’.

In the following, we introduce the intelligent driver model (IDM) (Treiber et al., 2000), which is a simple car-following model with intuitive parameters. Furthermore, we present three aspects of human driving behavior: (i) finite reaction times, (ii) temporal anticipation, and (iii) looking several vehicles ahead (spatial anticipation). These extensions are formulated in a systematic way and apply to all underlying models of the form (1) (Treiber et al., 2006).

2.1 The intelligent driver model (IDM)

The IDM acceleration of each vehicle $\alpha$ is a continuous function of the velocity $v_\alpha$, the net distance gap $s_\alpha$, and the velocity difference (approaching rate) $\Delta v_\alpha$ to the leading vehicle:

$$\frac{dv_\alpha}{dt} = a \left[ 1 - \left( \frac{v_\alpha}{v_0} \right)^4 - \left( \frac{s^*(v_\alpha, \Delta v_\alpha)}{s_\alpha} \right)^2 \right]. \quad (3)$$

The IDM acceleration consists of a free acceleration $\dot{v}^\text{free} = a [1 - (v/v_0)^4]$ (with $\dot{v}$ indicating the time derivative) for approaching the desired velocity $v_0$ with an acceleration slightly below $a$, and the braking interaction $\dot{v}^\text{int} = -a(s^*/s)^2$, where the actual gap $s_\alpha$ is compared with the ‘desired minimum gap’

$$s^*(v, \Delta v) = s_0 + vT + \frac{v\Delta v}{2\sqrt{ab}}, \quad (4)$$

which is specified by the sum of the minimum distance $s_0$, the velocity-dependent safety distance $vT$ corresponding to the time headway $T$, and a dynamic part. The dynamic part implements an accident-free ‘intelligent’ braking strategy that, in nearly all situations, limits braking decelerations to the ‘comfortable deceleration’ $b$. Notice that all five IDM parameters have an intuitive meaning. The parameters used henceforth (unless stated otherwise) are listed in Table 1. By an appropriate scaling of space and time, the number of parameters can be reduced from five to three.

2.2 Finite reaction time

A reaction time $T'$ is implemented simply by evaluating the right-hand side of Eq. (1) at time $t - T'$. If $T'$ is not a multiple of the update time interval, we propose a linear interpolation according to

$$x(t - T') = \beta x_{t-n-1} + (1 - \beta)x_{t-n}, \quad (5)$$

where $x$ denotes any quantity on the right-hand side of Eq. (1) such as $s_\alpha$, $v_\alpha$, or $\Delta v_\alpha$, and $x_{t-n}$ denotes this quantity taken $n$ time steps before the actual step. Here, $n$ is the integer part of $T'/\Delta t$, and the weight factor of the linear interpolation is given by $\beta = T'/\Delta t - n$. We emphasize that all input stimuli $s_\alpha$, $v_\alpha$, and $\Delta v_\alpha$ are evaluated at the delayed time.

Notice that the reaction time $T'$ is sometimes set equal to the ‘safety’ time-headway $T$. However, it is essential to distinguish between these times conceptually. While the time headway $T$ is a characteristic parameter of the driving style, the reaction time $T'$ is essentially a physiological parameter and, consequently, at most weakly correlated with $T$. We point out that both the time headway $T$ and the reaction time $T'$ are to be distinguished from the numerical update time step $\Delta t$, which is sometimes erroneously interpreted as a reaction time as well.

2.3 Temporal anticipation

We will assume that drivers are aware of their finite reaction time and anticipate the traffic situation accordingly. Besides anticipating the future situation (Davis, 2002), we will anticipate the future velocity using a constant-acceleration heuristics. The combined effects of a finite reaction time, and temporal anticipation lead to the following input variables for the underlying car-following model (1):

$$\frac{dv_\alpha}{dt} = \dot{v}^{\text{mic}}(s_\alpha', v_\alpha', \Delta v_\alpha') \quad (6)$$

with

$$s_\alpha'(t) = [s_\alpha - T'\Delta v_\alpha]_{t-T'}, \quad (7)$$
\[
v'_\alpha(t) = [v_\alpha + T'\dot{v}_\alpha]_{t-T'}, \quad (8)
\]
and
\[
\Delta v'_\alpha(t) = \Delta v_\alpha(t - T'). \quad (9)
\]
Notice that in Eq. (8) the time delay occurs in the acceleration \( \dot{v} \) as the highest derivative, i.e., the linearized model is of neutral type. We did not apply the constant-acceleration heuristics for the anticipation of the future velocity difference, or the future distance, as the accelerations of other vehicles cannot be estimated reliably by human drivers. Instead, we have applied the simpler constant-velocity heuristics for these cases. Notice that the proposed heuristics are parameter-free.

These ‘anticipative’ terms include derivative quantities (the accelerations), and velocity differences. In the framework of control theory, they act as nonlinear derivative elements in the control path.

2.4 Spatial anticipation for several vehicles ahead

Let us now split up the acceleration of the underlying microscopic model into a single-vehicle acceleration on a nearly empty road depending on the considered vehicle \( \alpha \) only, and a braking deceleration taking into account the vehicle-vehicle interaction with the preceding vehicle:

\[
\nu_{\text{mic}}^\alpha(s_\alpha, v_\alpha, \Delta v_\alpha) := \nu_\alpha^\text{free} + \nu_\alpha^\text{int}(s_\alpha, v_\alpha, \Delta v_\alpha). \quad (10)
\]

Next, we model the reaction to several vehicles ahead just by summing up the corresponding vehicle-vehicle pair interactions \( \nu_\alpha^\text{int} \) from vehicle \( \beta \) to vehicle \( \alpha \) for the \( n_a \) nearest preceding vehicles \( \beta \):

\[
\frac{dv_\alpha}{dt} = \nu_\alpha^\text{free} + \sum_{\beta=\alpha-n_a}^{\alpha-1} \nu_\alpha^\text{int}. \quad (11)
\]

where all distances, velocities and velocity differences on the right-hand side are given by Eqs. (7) - (9). Each pair interaction between vehicle \( \alpha \) and vehicle \( \beta \) is specified by

\[
\nu_{\alpha\beta}^\text{int} = \nu^\text{int}(s_{\alpha\beta}, v_\alpha, v_\alpha - v_\beta), \quad (12)
\]

where

\[
s_{\alpha\beta} = \sum_{j=\beta+1}^{\alpha} s_j \quad (13)
\]

is the sum of all net gaps between the vehicles \( \alpha \) and \( \beta \). For the IDM, there exists a closed-form solution of the multi-anticipative equilibrium distance as a function of the velocity. Notice that in the limiting case of anticipation to arbitrary many vehicles we obtain

\[
\lim_{n_a \to \infty} \gamma(n_a) = \pi/\sqrt{6} = 1.283 \text{ for the IDM. This means that the combined effects of all non-nearest-neighbor interactions would lead to an increase of the equilibrium distance by just about 28% (Treiber et al., 2006).}
\]

Table 1. Parameters of the intelligent driver model (IDM) with the values used in this paper unless stated otherwise. The IDM is used together with an explicit reaction time \( T' \) (cf. Sec. 2.2), temporal anticipation (cf. Sec. 2.3), and spatial anticipation (cf. Sec. 2.4). The vehicle length is 5 m. Furthermore, we restrict the maximum braking deceleration to 9 m/s² as the physical limit on dry roads.

<table>
<thead>
<tr>
<th>Parameter</th>
<th>Value</th>
</tr>
</thead>
<tbody>
<tr>
<td>Desired velocity ( v_0 )</td>
<td>120 km/h</td>
</tr>
<tr>
<td>Save time headway ( T )</td>
<td>1.5 s</td>
</tr>
<tr>
<td>Jam distance ( s_0 )</td>
<td>2 m</td>
</tr>
<tr>
<td>Maximum acceleration ( a )</td>
<td>2.0 m/s²</td>
</tr>
<tr>
<td>Desired deceleration ( b )</td>
<td>2.0 m/s²</td>
</tr>
</tbody>
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3. MICROSCOPIC TRAFFIC SIMULATIONS OF VEHICLE PLATOONS

We investigate the string stability by simulating a platoon of vehicles following an externally controlled lead vehicle. As initial conditions, we assume the platoon to be in equilibrium, i.e., the initial velocities of all platoon vehicles are equal to \( v_{\text{lead}} \) and the gaps equal to \( s_n(v_{\text{lead}}) \) (cf. Eq. (2)), so that the initial model accelerations are equal to zero.

The externally controlled vehicle drives at \( v_{\text{lead}} = 25 \text{ m/s} \) for the first 1000 s, before it decelerates with \(-2 \text{ m/s}^2 \) for 3 s, which is a realistic scenario in daily traffic situations. This braking maneuver reduces the velocity to \( v_{\text{lead}} = 19 \text{ m/s} \), which is kept constant until the simulation ends at \( t = 2500 \text{ s} \). Figure 1 shows the time series of the acceleration and velocity of the lead vehicle and the distance to the lead vehicle of the second car in the platoon. This braking maneuver serves as perturbation for all simulations throughout this paper. Notice that the nonlinear dynamics resulting from this strong perturbation cannot be handled by linearization anymore.

In all simulations, we have used an explicit integration scheme assuming constant accelerations between each update time interval \( \Delta t \) according to

\[
v_\alpha(t + \Delta t) = v_\alpha(t) + \dot{v}_\alpha(t)\Delta t, \quad (14)
\]

\[
x_\alpha(t + \Delta t) = x_\alpha(t) + v_\alpha(t)\Delta t + \frac{1}{2} \dot{v}_\alpha(t)(\Delta t)^2.
\]

The update time interval is set to \( \Delta t = 0.1 \text{ s} \). We will use the IDM parameters given in Table 1 unless stated otherwise. If \( n_a \) is larger than the number of preceding vehicles (which can happen for the first vehicles of the platoon) then \( n_a \) is reduced accordingly. Furthermore, we restrict the maximum braking deceleration to 9 m/s², which is the physical limit on dry roads.
the oscillatory regime.

Figure 2 shows the three stability regimes as a function of the reaction time $T'$ and the platoon size $n$ for the following simulation scenarios:

1. The first scenario without neither spatial anticipation ($n_a = 1$) nor temporal anticipation serves as reference. This case corresponds to the conventional IDM car-following model with finite reaction time (cf. Sec. 2.2). A platoon of 100 vehicles is stable for reaction times of up to $T'_{c1} = 0.9\,\text{s}$. Test runs with larger platoon sizes (up to 1000 vehicles) did not result in different thresholds suggesting that stability for a platoon size of 100 essentially means string stability for arbitrarily large platoon sizes. Reaction times $T' > T'_{c2} = 1.15\,\text{s}$ lead to collective instability in combination with the applied braking limit of $9\,\text{m/s}^2$.

2. The second scenario extends the reference scenario by implementing the parameter-free temporal anticipation (cf. Sec. 2.3), which leads to an increased stability, particularly for the second phase boundary $T'_{c2}$.

3. The third simulation scenario implements the spatial anticipation of looking $n_a = 4$ vehicles ahead (cf. Sec. 2.4) as extension compared to the reference case. This anticipation increases the stability and shifts both boundaries, $T'_{c1}$ and $T'_{c2}$, to higher values.

4. The fourth scenario combines temporal and spatial anticipation ($n_a = 4$), which leads to the most stable system. Particularly, the second boundary is shifted to values of $T'_{c2} \geq 2\,\text{s}$. Remarkably, the simulation shows that, with a suitable anticipation, we could obtain string stability for reaction times exceeding the "safe time headway" of $T = 1.5\,\text{s}$.

So far, we have assumed constant and identical reaction times $T'_0 = T'$ for all vehicles $\alpha$ in the simulation. Since the human reaction time varies strongly depending on the concrete situation and between different persons (Green, 2000), we also investigate the role of distributed reaction times, i.e., every driver has a different reaction time $T'_\alpha$ with the mean value $(T'_0) = T'$. To this end, we generalize the concept of linear interpolation of Eq. (5) to individual delays for each driver-vehicle unit $\alpha$.

Figure 3 shows the simulation results for the reference scenario (1) without anticipation and the scenario (4) with temporal and spatial anticipation ($n_a = 4$). The reaction time has been uniformly distributed within a range of ±30% around the mean value. Interestingly, the phase boundary $T'_{c1}$ between the stable and oscillatory regime is nearly not affected by the variation of the reaction time. Remarkably, the phase boundary $T'_{c1}$ for the forth scenario is even slightly shifted towards higher stability for platoon sizes of $n \leq 60$ vehicles. However, the critical value $T'_{c2}$ is slightly reduced when dealing with non-identical reaction times.
Fig. 2. String stability regimes of a platoon of identical vehicles as a function of the platoon size and the reaction time $T'\tau$ for the scenarios (1) - (4) described in Sec. 3.1. The graph (a) depicts scenario (1) assuming conventional follow-the-leader behavior ($n_a = 1$) without temporal anticipation; (b) with temporal anticipation ($n_a = 1$) (scenario (2)); (c) reaction to $n_a = 4$ vehicles without temporal anticipation (scenario (3)); (d) reaction to $n_a = 4$ vehicles with temporal anticipation (scenario (4)). In the diagrams (b)-(d), the first scenario of graph (a) is plotted with thin lines for purposes of comparison. The externally controlled first vehicle induced a perturbation according to Fig. 1. In the ‘stable’ phase, all perturbations are damped away. In the oscillatory regime, the perturbations increase, but do not lead to crashes.

Fig. 3. String stability regimes of a platoon of vehicles $\alpha$ with different individual reaction times. The reaction time $T'_\alpha$ has been distributed uniformly within 30% around the mean value $\langle T' \rangle = T'$. The diagram (a) refers to the scenario (1) without temporal nor spatial anticipation, while the diagram (b) corresponds to the forth scenario with temporal and spatial anticipation ($n_a = 4$). The thin lines display the phase boundaries for identical reaction times. The variation of the reaction times leads to a nearly unchanged (see (a)) or even slightly increased (see (b)) stability boundary $T'_{c1}$ between the stable and oscillating regime.
Fig. 4. Time series of the acceleration for selected platoon vehicles. The reaction time is $T_0 = 0.9$ s, and the IDM parameter for the maximum acceleration is set to $a = 1$ m/s$^2$. The vehicles use temporal anticipation but no spatial anticipation ($n_a = 1$). The first vehicle induces a perturbation due to the braking maneuver at $t = 1000$ s. The initial perturbation do not increase while propagating through the platoon of vehicles. The system is string stable.

3.2 Role of vehicle acceleration

As mentioned in the introduction, there are basically two different sources of instability for traffic flow: The finite reaction time modelled by the HDM parameter $T'$, and finite acceleration capabilities modelled by the IDM parameter $a$, which gives the maximum acceleration.

Clearly, stability always decreases when $T'$ increases. In this subsection, we investigate how the acceleration parameter $a$ influences the instability mechanisms and come to the remarkable result that stability reaches its maximum for a certain range of values for $a$ (that depends on $T'$). Traffic flow becomes more unstable if the value of $a$ is higher or lower than this range.

Figure 4 shows time series of the acceleration of some selected vehicles for scenario (1) with $T' = 0.9$ s, and the acceleration parameter changed from 2 m/s$^2$ to the approximatively 'optimal' value 1 m/s$^2$. The system is string stable: the initial perturbation of 2 m/s$^2$ dissipates quickly.

In Figure 5, the acceleration parameter is lowered from 1 m/s$^2$ to $a = 0.3$ m/s$^2$. The effect is as expected (Treiber et al., 2000; Helbing, 2001; Treiber et al., 1999): The initial perturbation decreases for the first few vehicles (the system is locally stable), before it increases again for the next vehicles, and finally leads to a traffic breakdown in the neighborhood of vehicle 100 at a simulated time $t ≈ 1250$ s: The system is string unstable. After the first breakdown, further stop-and-go waves develop (Fig. 6).

Remarkably, the system becomes unstable as well when increasing the acceleration capability from the reference value 1 m/s$^2$ to $a = 2.5$ m/s$^2$ as shown in Fig. 7. The instability mechanism, however, is different. For low values of $a$, the traffic breakdown is initially triggered by a long-wavelength instability as can be seen in Fig. 5 in the plots for the cars 10 and 50, before additional shorter oscillations appear immediately before breakdown (vehicles 80 and 100). In contrast, the initial instability for high values of $a$ has its maximum growth rates at shorter frequencies.
Fig. 6. Patterns of stop-and-go waves. The simulation is identical to that of Fig. 5, but vehicles further upstream are shown on a different time scale. The period of the stop-and-go waves is about 40s.

(of about 4 s), which can be seen from Fig. 7 for the vehicle sequence 4, 10, and 20 leading to the first stop-and-go wave, and the sequence 50, 60, 70, leading to the second one. Further stop-and-go waves develop at later times for vehicles further upstream. Interestingly, the period of the resulting stop-and-go waves is about the same for the high-wavelength, and low-wavelength mechanisms to instability.

Fig. 7. Time series of the acceleration for the same scenario as in Fig. 4, but the IDM parameter for the maximum acceleration is increased to $a = 2.5 \text{ m/s}^2$. Again, the first vehicle induces a perturbation due to the braking maneuver at $t = 1000 \text{ s}$ (not shown here). The increased acceleration parameter $a$ in combination with the delayed reaction causes higher frequencies with periods about 4s that finally trigger stop-and-go-waves of a much higher period (about 50s).
In this contribution, we have investigated two causes for the instability of traffic flow, the time lag caused by finite accelerations of the vehicles, and the delay caused by the finite reaction time of the drivers. Furthermore, we have simulated to which degree drivers may compensate for these delays by looking several vehicles ahead and anticipate future traffic situations.

Since vehicular traffic flow is a multi-particle system with many degrees of freedom, two concepts of linear stability have to be considered: Local stability of a car following a leader that drives at constant velocity, and string or collective stability of a platoon of several vehicles following each other. Typically, string stability is a much more restrictive criterion than local stability.

Our main results are:

(i) By means of simulation, we determined the string stability boundaries as a function of the reaction time \(T'\) for a variable platoon size of vehicles. With a suitable spatial and temporal anticipation, we obtained string stability for reaction times exceeding the "safe time headway", which, to date, has not yet been obtained for any other car-following model.

(ii) When varying the maximum acceleration capability, we come to the remarkable result that stability reaches its maximum for a certain range of values for \(a\) (that depends on the reaction time \(T'\)). Traffic flow becomes more unstable if the value of the maximum acceleration is higher or lower than this value. This can be understood by the interplay between the two mechanisms to instabilities: If the value of \(T'\) and \(a\) are both comparatively high, then the ratio between the reaction time and the time scale \(\tau \approx v_0/a\) of velocity changes is high leading to instabilities on the level of individual vehicles. Conversely, for low values of \(a\), the lag time scale \(\tau\) itself leads to the well-known collective instabilities already observed for zero reaction time.

(iii) Distributed reaction times, i.e., every driver has a different reaction time, can stabilize the system compared to drivers with identical reaction times that are equal to the mean. Generally, however, the effect introduced by the heterogeneity among drivers is small.

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